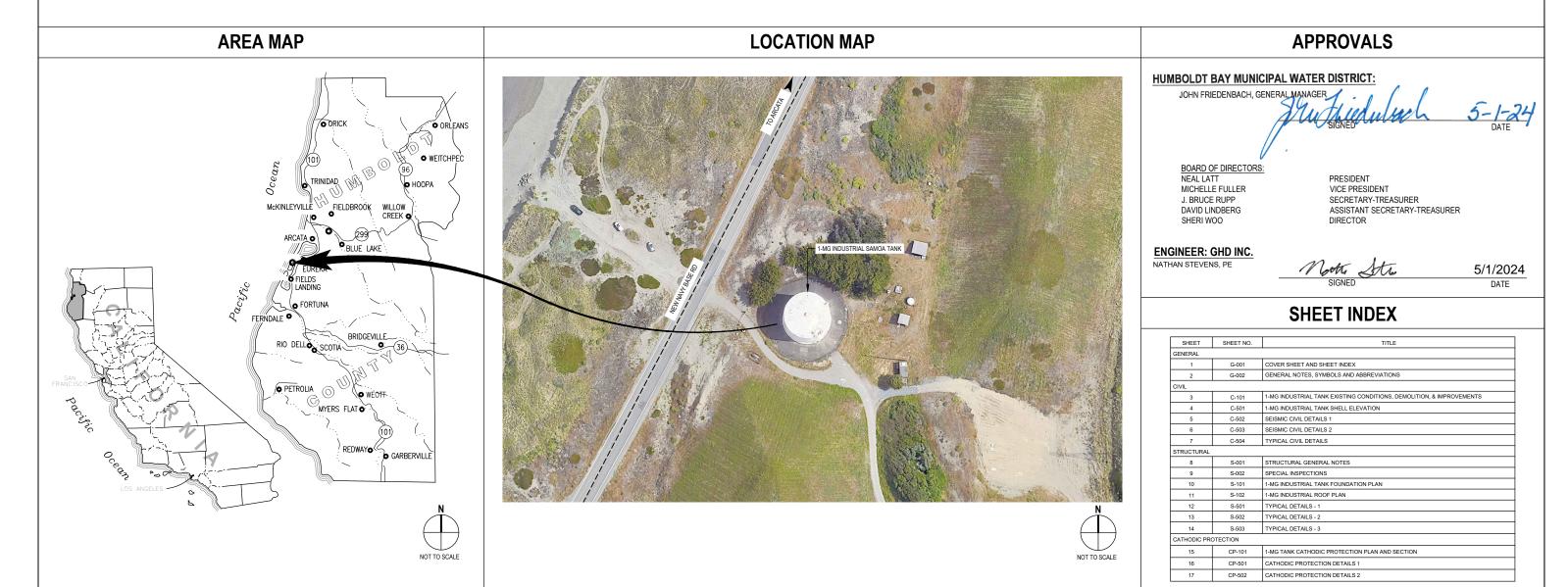
HUMBOLDT BAY MUNICIPAL WATER DISTRICT

SAMOA RESERVOIR SEISMIC RETROFIT PROJECT

MAY 2024 PREPARED BY:





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Designer M. DUIN

Design Check S. MCHANEY Project Director K. TOBIN

* HUMBOLDT BAY MUNICIPAL WATER

2024-05-01

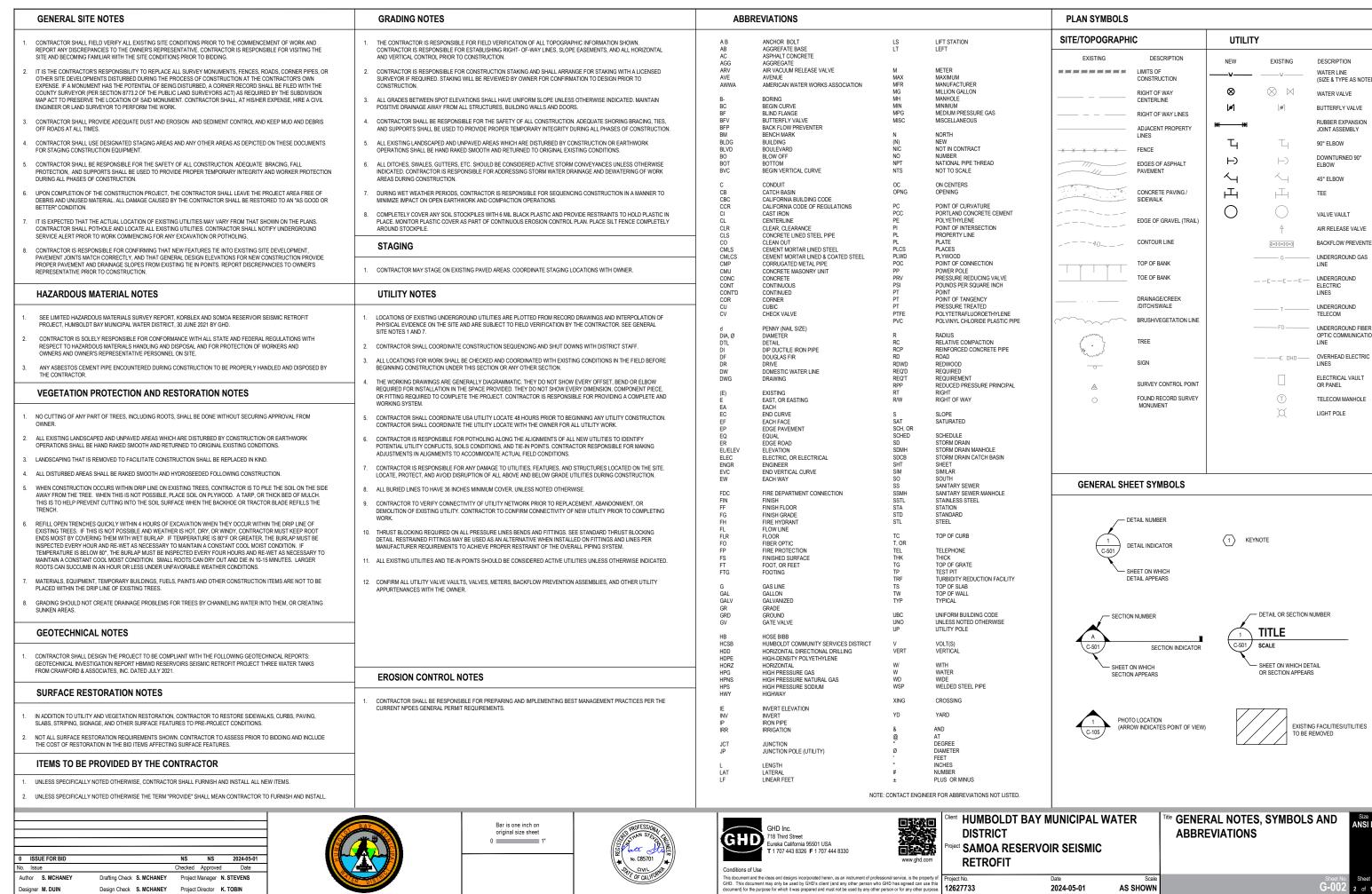
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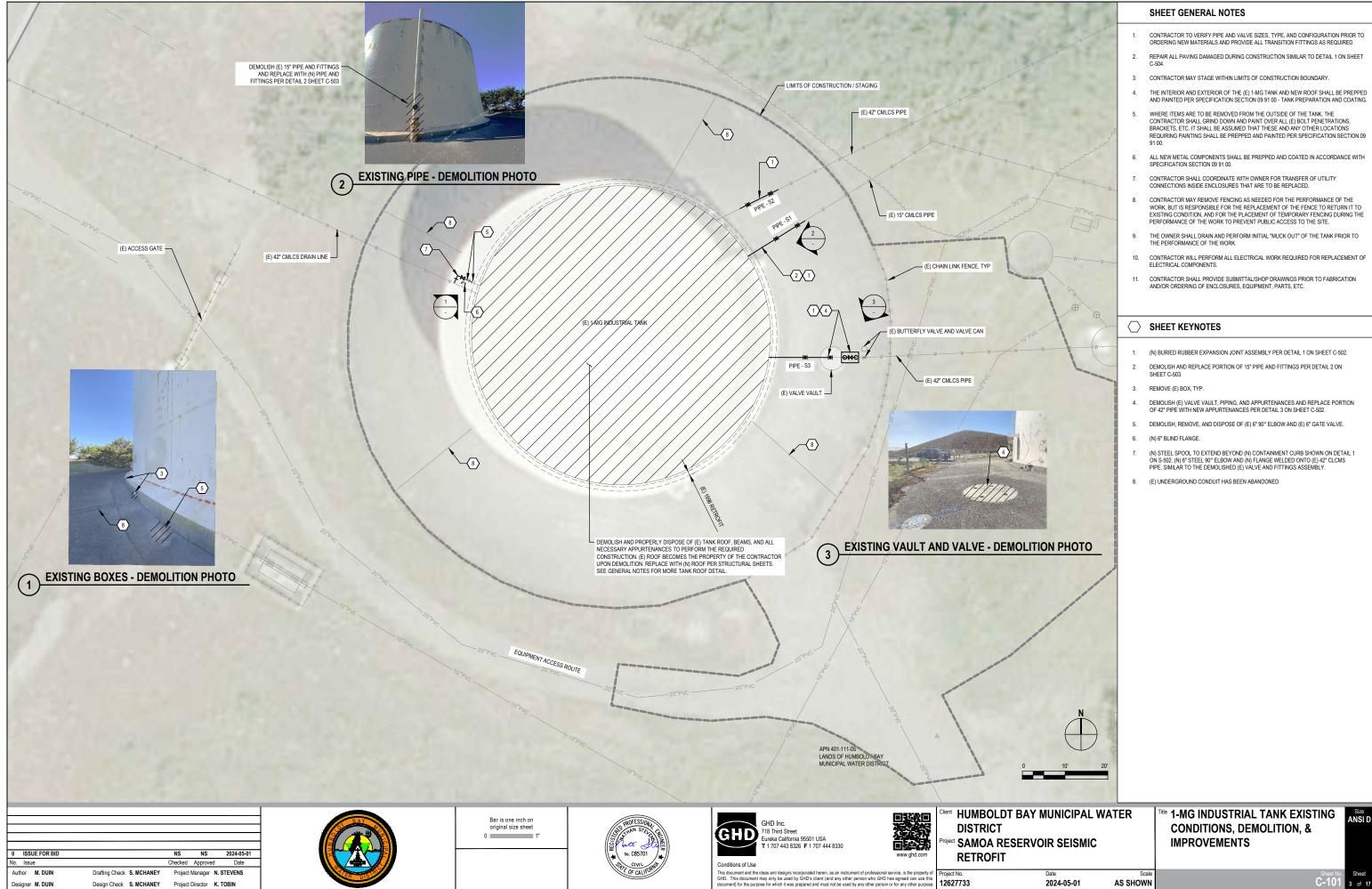
* SAMOA RESERVOIR SEISMIC

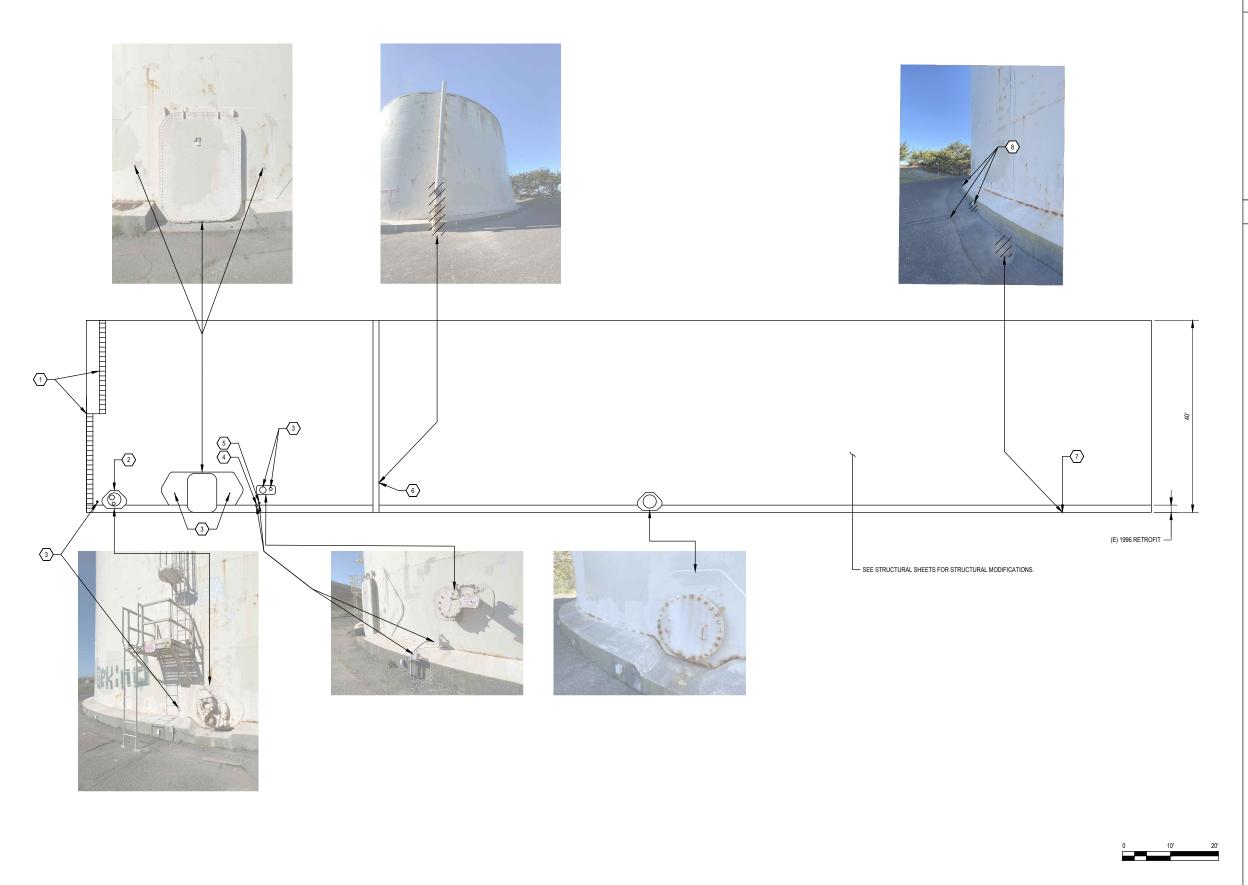
DISTRICT

RETROFIT

COVER SHEET AND SHEET INDEX







SHEET GENERAL NOTES

- WHERE ITEMS ARE TO BE REMOVED FROM THE OUTSIDE OF THE TANK, THE CONTRACTOR SHALL GRIND DOWN ALL (E) BOLT PENETRATIONS, BRACKETS, ETC. AND WELD 1/4* STEEL PLATES OVER OPENINGS. IT SHALL BE ASSUMED THAT THESE AND ANY OTHER LOCATIONS REQUIRING PAINTING SHALL BE PREPPED AND PAINTED PER SPECIFICATION SECTION 09 91 00.
- ALL NEW METAL COMPONENTS SHALL BE PREPPED AND COATED IN ACCORDANCE WITH SPECIFICATION SECTION 09 91 00.
- CONTRACTOR SHALL PROVIDE SUBMITTAL/SHOP DRAWINGS PRIOR TO FABRICATION AND/OR ORDERING OF ENCLOSURES, EQUIPMENT, PARTS, ETC.
- FOR ALL HATCHES, COVERS AND FITTINGS REMOVED DURING PROJECT, REPLACE GASKETS AND BOLTS, NUTS, AND WASHERS.
- ALL ABOVE GROLIND HARDWARE TO BE HOT DIPPED GALVANIZED LINLESS NOTED OTHERWISE. ALL BELOW GROUND BOLTS, NUTS AND WASHERS TO BE STAINLESS STEEL.
- WELD STEEL PLATES TO THE INTERIOR AND EXTERIOR OF THE TANK WHERE STEEL PLATES ARE REQUIRED USING 3/8" FILLET WELD ALL AROUND.

- 1. DEMOLISH (E) LADDER AND CAGE ASSEMBLY. INSTALL (N) LADDER, (N) CAGE, AND (N) PLATFORM PER DETAIL 6 ON SHEET S-501. DEMOLISH AND REPLACE (E) INTERIOR LADDER WITH (N) COATED STEEL LADDER.
- REPLACE (E) 8" AND 6" VALVES WI (N) VALVES WITH OPERATING NUT. CONFIRM ORIENTATION IN FIELD WITH OWNER.
- 3. CUT (E) FEATURE FLUSH AT TANK FACE AND WELD PLATE. SEE SHEET GENERAL NOTES 1
- 4. IF REQUIRED, RELOCATE (E) ELECTRICAL CONDUIT AND PULL BOX OUTSIDE OF STRUCTURAL MODIFICATIONS.
- IF REQUIRED, EXTEND (E) TAP WITH COATED STEEL PIPING PAST STRUCTURAL MODIFICATIONS AND REINSTALL (E) PRESSURE TRANSDUCER OUTSIDE STRUCTURAL MODIFICATIONS.
- 6. DEMOLISH AND REPLACE PORTION OF 15" PIPE AND FITTINGS PER DETAIL 2 ON SHEET
 - DEMOLISH AND REPLACE PORTION OF 6" DRAIN LINE, VALVES, FITTINGS, AND VALVE CAN
- 8. REMOVE (E) BOX, TYP. (E) UNDERGROUND CONDUIT HAS BEEN ABANDONED.



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12627733

Design Check S. MCHANEY Project Director K. TOBIN

NS NS 2024-05-01

Date

Checked Approved

Drafting Check S. MCHANEY Project Manager N. STEVENS

0 ISSUE FOR BID

Author M. DUIN

Designer M. DUIN

Plot Date: 30 April 2024 - 9:29 PM

No. Issue

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AS SHOWN

2024-05-01

Title 1-MG INDUSTRIAL TANK SHELL **ELEVATION**

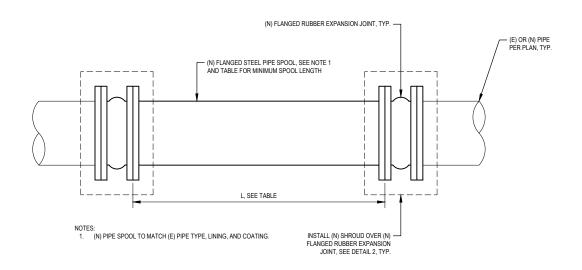


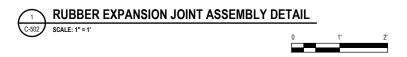


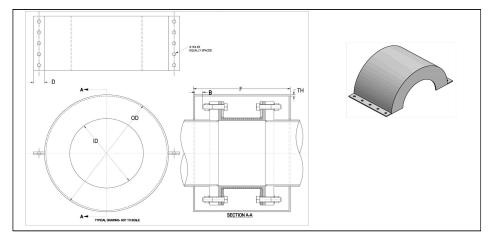


	EXPANSION JOINT ASSEMBLY FOR UP TO 6" OF SETTLEMENT								
PIPE DESIGNATION	EXISTING PIPE DIAMETER ¹	EXISTING PIPE MATERIAL ¹	NEW FLEX COUPLING DIAMETER	FLANGES NEEDED ON EXISTING PIPE	TRANSITION FITTING REQUIRED ²	SHROUD REQUIRED ³	MINIMUM SPOOL LENGTH (L _{MIN})		
S1	15"	CMLCS	16"	YES	YES	YES	60"		
S2	42"	CMLCS	42"	YES	NO	YES	100"		
S3	42"	CMLCS	42"	YES	NO	YES	100"		

- (1) CONTRACTOR TO FIELD VERIFY EXISTING SIZES, MATERIALS, AND TRANSITION REQUIREMENTS PRIOR TO ORDERING MATERIALS.
 (2) SIMILAR TO TRANSITION FITTING ON DETAIL 2 ON SHEET C-503.
- (3) SEE DETAIL 2 ON THIS SHEET.

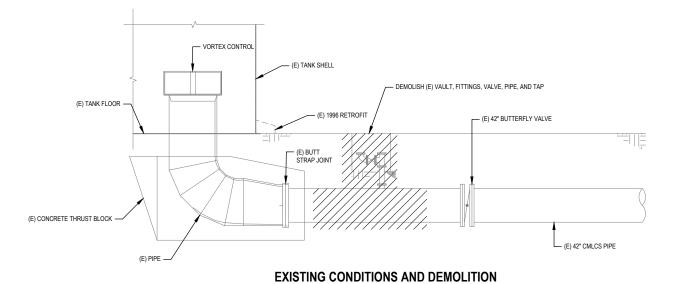


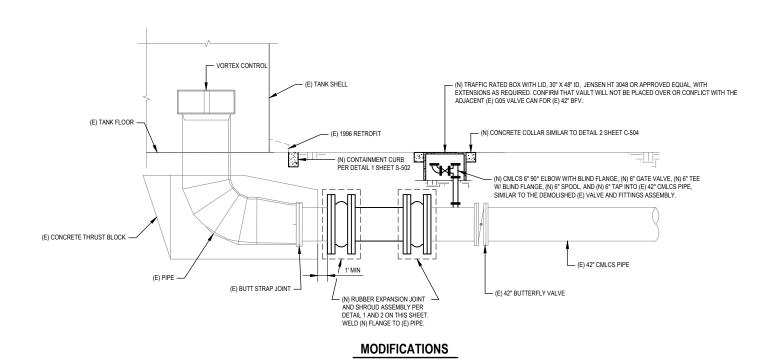


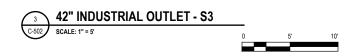


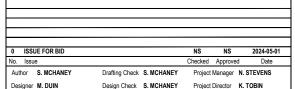
- NOTES:
 1. SHROUD SHALL BE DESIGNED, FABRICATED AND INSTALLED PER THE FLEXIBLE COUPLER MANUFACTURER INSTRUCTIONS.
- 2. SHROUD SHALL BE COATED PER MANUFACTURER RECOMMENDATION. ALL HARDWARE TO BE STAINLESS STEEL.



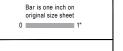


















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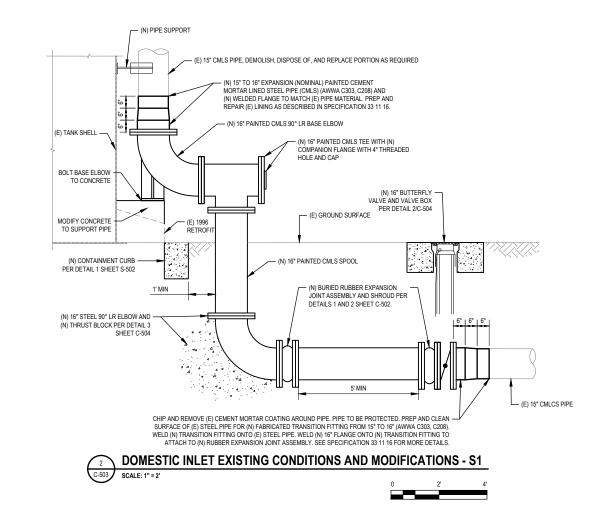
2024-05-01

AS SHOWN

Title SEISMIC CIVIL DETAILS 1

Size ANSI D





0 ISSUE FOR BID NS NS 2024-05-01 No. Issue Checked Approved Date Author S. MCHANEY Drafting Check S. MCHANEY Project Manager N. STEVENS Designer M. DUIN Design Check S. MCHANEY Project Director K. TOBIN Plot Date: 30 April 2024 - 9:31 PM



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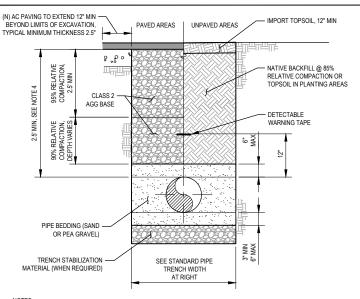
Client	HUMBOLDT BAY MUNICIPAL WATER
	DISTRICT
Project	SAMOA RESERVOIR SEISMIC
	RETROFIT

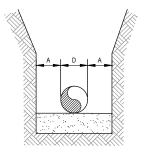
2024-05-01

AS SHOWN

Title SEISMIC CIVIL DETAILS 2

Size ANSI D



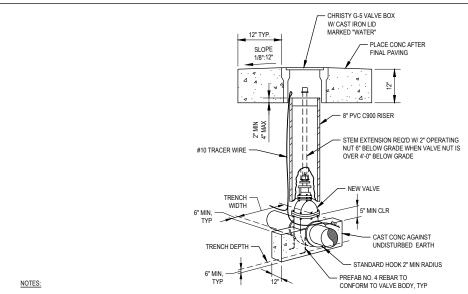


PIPE DIA "D"	MINIMUM "A"	MAXIMUM "A"
< 4"	3"	6"
4" TO 6"	6"	12"
6" TO 15"	8"	14"
16" TO 21"	10"	16"
24" TO 30"	12"	18"
33" TO 42"	15"	21"
48" & LARGER	18"	24"

NOTES:

- 1. WIDER TRENCHES MAY REQUIRE HIGHER STRENGTH PIPE AND/OR SPECIAL BEDDING.
- 2. DIFFERING TRENCH WIDTHS REQUIRE PRIOR APPROVAL OF ENGINEER.
- 3. IN MAKING EXCAVATIONS FOR THIS PROJECT. THE CONTRACTOR SHALL BE FULLY RESPONSIBLE FOR PROVIDING & INSTALLING ADEQUATE SHEETING. SHORING & BRACING AS MAY BE NECESSARY AS A PRECAUTION AGAINST SLIDES OR CAVE-INS, AND TO PROTECT ALL (E) IMPROVEMENTS OF ANY KIND, EITHER ON PUBLIC OR PRIVATE PROPERTY, FULLY FROM DAMAGE.
- 4. 2-SACK SLURRY BACKFILL MAY BE USED IN TRENCH WHEN MINIMUM PIPE COVER NOT POSSIBLE, WHEN APPROVED BY OWNER'S REPRESENTATIVE.
- 5. CLASS 2 AGGREGATE BASE SHALL BE COMPACTED TO 95% RELATIVE COMPACTION.
- 6. DETECTABLE WARNING TAPE SHALL BE BRIGHT COLORED, CONTINUOUSLY PRINTED, MINIMUM 6" WIDE BY 4 MIL. THICK, MANUFACTURED FOR DIRECT BURIAL.





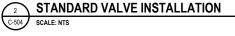
NS NS 2024-05-01

Project Manager N. STEVENS

Date

Checked Approved

- 1. CONTRACTOR TO PROVIDE ALL COMPONENTS.
- 2. VALVE SIZES & ENDS AS SHOWN OR SPECIFIED ON PLANS





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HUMBOLDT BAY MUNICIPAL WATER DISTRICT * SAMOA RESERVOIR SEISMIC

RETROFIT 2024-05-01 AS SHOWN

Title TYPICAL CIVIL DETAILS

Size ANSI D

STANDARD VALVE INSTALLATION

2. KEEP CONCRETE CLEAR OF JOINTS AND ACCESSORIES.

BEARING AREA = (TEST PRESSURE / 150) x

CONTRACTOR TO PROVIDE ALL COMPONENTS.

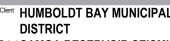
(1000 / SOIL BEARING STRESS) x (TABLE VALUE)

THRUST BLOCKS SHALL NOT BE LOCATED OR SIZED TO ENCASE ADJACENT PIPES OR FITTINGS.

THE SIZE AND WEIGH OF ALL UPLIFT THRUST BLOCKS SHALL BE AS DETERMINED BY ENGINEER.

THE BEARING AREAS ARE BASED ON TEST PRESSURE OF 150 PSI AND ALLOWABLE SOIL BEARING STRESS OF 1000 POUNDS PER SQUARE FOOT. TO COMPUTE BEARING AREAS FOR DIFFERENT TEST PRESSURES AND SOIL BEARING STRESSES, USE THE FOLLOWING EQUATION:

ALL PIPE AND FITTINGS SHALL BE WRAPPED IN POLYETHYLENE TO PREVENT CORROSION AND CONC



2' MIN, TYP 3' MIN WHEN LENGTH IS GREATER THAN 15'

TYPICAL SECTION

0 ISSUE FOR BID

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Designer M. DUIN

No. Issue

Design Check S. MCHANEY Project Director K. TOBIN

Drafting Check S. MCHANEY

CONCRETE THRUST BLOCKS ARE TO BE POURED AGAINST UNDISTURBED EARTH OR STRUCTURAL ${\tt BACKFILL.THRUST\ BLOCKS\ ARE\ NOT\ REQUIRED\ WHERE\ JOINTS\ ARE\ ADEQUATELY\ RESTRAINED.}$ VOLUMES AND SPECIAL BLOCKING DETAILS SHOWN ON THE PLANS TAKE PRECEDENCE OVER VOLUMES AND BLOCKING DETAILS SHOWN ON THIS STANDARD DETAIL. ALL BURIED PIPE SHALL BE PROVIDED WITH CONCRETE THRUST BLOCKS AT ALL DIRECTIONAL CHANGES 2500 PSI MIN CONC. USE STEEL WHERE LENGTH IS 9' OR MORE

ELEVATION

	BEARING AREA OF THRUST BLOCK IN SQ. FT.															
PIPE SIZE	TEE, WYE, PLUG OR	90° BEND PLUGGED		EE GGED		11 1/4° BEND		PIPE SIZE		90° BEND PLUGGED	TEE PLUGGED		45° BEND	22 1/2° BEND	11 1/4° BEND	
OILL	CAP	CROSS	A1	A2					OILL	CAP	CROSS	A1	A2			
4	1.5	2	2	1.5	1.5	1	1		18	19	27	27	19	15	8	6
6	3	4.5	4.5	3	2.5	1.5	1		20	24	34	34	24	18	10	8
8	5	7	7	5	4	2	1		22	29	41	41	29	22	12	10
10	8	12	12	8	7	3	2		24	34	48	48	34	26.5	14	12
12	12	17	17	12	10	5	3		32	39	55	55	39	31.5	16	14
16	15	21.5	21.5	15	12	6	4									

(3-#5 HORIZ/ #4 AT 18" VERT

STANDARD THRUST BLOCK DETAILS

SHEET GENERAL NOTES STEEL CONCRETE 1. CONTRACTOR TO COORDINATE ALL STRUCTURAL DOCUMENTS WITH ALL OTHER DISCIPLINES AND REPORT ANY DETAIL, FABRICATE, AND ERECT STRUCTURAL STEEL IN ACCORDANCE WITH THE AMERICAN INSTITUTE OF STEEL ALL CONCRETE SHALL BE NORMAL WEIGHT. WITH A MINIMUM COMPRESSIVE STRENGTH OF 5000 PSI AT 28 DAYS. DISCREPANCIES TO THE OWNER PRIOR TO THE START OF ANY FABRICATION OR CONSTRUCTION CONSTRUCTION SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS (LATEST EDITION AND SUPPLEMENTS CONCRETE REINFORCING COVER SHALL BE AS FOLLOWS: ANCHOR BOLTS: ASTM F1554 GRADE 55. 2. CONTRACTOR TO COORDINATE ALL NEW WORK WITH EXISTING SITE CONDITIONS AND REPORT ANY DISCREPANCIES TO CONCRETE CAST AGAINST AND PERMANENTLY EXPOSED TO THE OWNER PRIOR TO CONSTRUCTION ALL STEEL BARS & PLATES SHALL BE ASTM A36 UNLESS OTHERWISE NOTED. CONCRETE EXPOSED TO EARTH OR WEATHER2 INCHES DO NOT SCALE DRAWINGS. 4. ALL STEEL SHAPES SHALL BE ASTM A992 GRADE 50 UNLESS OTHERWISE NOTED. ALL CONCRETE DIMENSIONS SHOWN ARE MINIMUM DIMENSIONS. CONTRACTOR TO REVIEW FORMING, REINFORCING DETAILS AND ANY EMBEDDED ITEMS AND DETERMINE PRIOR TO FABRICATION OF ANY 4. DESIGN CRITERIA: 2019 CALIFORNIA BUILDING CODE (2019 CBC) ALL TUBES SHALL BE ASTM A500 GRADE B. AWWA D100-11 REINFORCING, PLACEMENT REQUIREMENTS AND CLEARANCES. ALL PIPES TO BE ASTM A53 GRADE B. ACI 318-14 4. EPOXY ANCHORS SHALL BE ONE OF THE FOLLOWING, UNO: CAL / OSHA ALL THREADED RODS: ASTM F1554 GRADE 55. LOADS: HILTLHIT-HY 200 (ICC-ES REPORT ESR-3187) BOLTED CONNECTIONS, UNLESS NOTED OTHERWISE: 1-INCH DIAMETER A325-N BOLTS. ROOF LIVE LOADS: 20 PSF (REDUCTIONS TAKEN AS ALLOWED BY BUILDING CODE) HILT HIT-RE 500 (ICC-ES REPORT ESR-2322) MAINTENANCE PLATFORM: INSTALL HIGH STRENGTH BOLTS IN ACCORDANCE WITH SECTION 8 OF THE "SPECIFICATIONS FOR STRUCTURAL JOINTS WIND LOADS USING ASTM A325 OR A490 BOLTS", LATEST EDITION. SIMPSON SET-3G (ICC-ES REPORT ESR-4057) MAIN FORCE RESISTING SYSTEM: PROVIDE BEVELED WASHERS ON ALL CONNECTION TO SLOPING FLANGES OF W SECTIONS AND CHANNELS WHERE BASIC WIND SPEED: V = 115 MPH SLOPE EXCEEDS 1:20. RISK CATEGORY IV (ESSENTIAL FACILITY) EXPOSURE CATEGORY ANCHOR RODS SHALL BE THREADED ANCHOR RODS WITH NUT. THE EMBEDDED NUT SHALL BE TACK WELDED TO THE INTERNAL PRESSURE COEFFICIENT: ANCHOR ROD TO PREVENT ROTATION DURING TIGHTENING ±0.18 SEISMIC LOADS (SAMOA): BOLT HOLES IN STEEL SHALL BE "STANDARD" (1/16-INCH LARGER IN DIAMETER THAN THE NOMINAL BOLT SIZE), UNLESS SEISMIC IMPORTANCE FACTOR: IE = 1.50 OTHERWISE NOTED MAPPED SPECTRAL RESPONSE ACCELERATIONS: WELDING ELECTRODES (FILLER METAL): E70XX (70 KSI), WITH EXACT FILLER METAL SELECTED BY THE FABRICATOR. Ss = N/A S₁ = N/A WELD LENGTHS CALLED FOR ON THE PLANS ARE THE NET EFFECTIVE LENGTH REQUIRED. WHERE LENGTH OF WELD SPECTRAL RESPONSE COEFFICIENTS: IS NOT SHOWN IT SHALL BE THE FULL LENGTH OF THE JOINT REINFORCING SDS = 1.21 g COMPLETE PENETRATION WELDS SHALL BE MADE WITH PROPER BACKING WHEREVER POSSIBLE. FULL PENETRATION SD1 = 1.87 a SOIL SITE CLASS: F WELDS MADE WITHOUT PROPER BACKING SHALL HAVE THE ROOT GOUGED BEFORE WELDING IS STARTED FROM THE ALL CONCRETE REINFORCING SHALL BE ASTM A615, Fy = 60 KSI., UNLESS NOTED OTHERWISE. OTHER SIDE EXCEPT AS PROVIDED IN AWS D1.1. SEISMIC DESIGN CATEGORY: F REINFORCING SHALL EXTEND CONTINUOUS FOR THE DIMENSION SHOWN ALL BUTT AND GROOVE WELDS SHALL BE FULL PENETRATION, UNLESS NOTED OTHERWISE. REFERENCE TO CODES, RULES, REGULATIONS, STANDARDS, MANUFACTURER'S INSTRUCTIONS OR REQUIREMENTS OF REGULATORY AGENCIES IS TO THE LATEST PRINTED EDITION OF EACH IN EFFECT AT THE DATE OF SUBMISSION OF BID UNLESS THE DOCUMENT DATE IS SHOWN. NO WELDING OF ANY REINFORCING IS PERMITTED, UNLESS SPECIFICALLY STATED ON THE PLANS. REINFORCEMENT ALL SPLICING OF MEMBERS SHALL BE AS SHOWN ON THE DRAWINGS. ANY SPLICING OF THE STEEL MEMBERS PROPOSED BY THE STEEL FABRICATOR SHALL BE SHOWN ON SHOP DRAWINGS AND APPROVED BY THE ENGINEER TO BE WELDED TO MEET THE REQUIREMENTS OF ASTM A706. PRIOR TO FARRICATION LOCATE ALL REINFORCING AS SHOWN ON DRAWINGS AND FASTEN SECURELY. THESE DRAWINGS INDICATE GENERAL AND TYPICAL DETAILS OF CONSTRUCTION. WHERE CONDITIONS ARE NOT SPECIFICALLY INDICATED BUT ARE OF SIMILAR CHARACTER TO DETAILS SHOWN, USE SIMILAR DETAILS OF CONSTRUCTION, SUBJECT TO REVIEW BY THE OWNER'S REPRESENTATIVE. LAP SPLICES AND DEVELOPMENT LENGTHS PER DETAIL ON DRAWING S-501. ALL STEEL FABRICATION SHALL BE PERFORMED BY A FABRICATOR APPROVED BY THE OWNER. REINFORCEMENT SHALL BE PLACED SO AS NOT TO COME IN CONTRACT WITH METALLIC CONCRETE PENETRATIONS ALL ANCHOR BOLTS SHALL BE EMBEDDED AS SHOWN ON THE DRAWINGS. THE CONTRACTOR IS RESPONSIBLE FOR COORDINATING THE WORK OF ALL TRADES AND FOR CHECKING DIMENSIONS. LL REINFORCING TO TERMINATE WITH STANDARD HOOKS AS SHOWN ON PLANS. ALL STIRRUPS AND TIES TO BE NOTIFY THE OWNER'S REPRESENTATIVE OF ANY DISCREPANCIES AND RESOLVE BEFORE PROCEEDING WITH THE WORK 20. MINIMUM PLATE THICKNESS: 3/8 INCH UNLESS OTHERWISE NOTED. MINIMUM WELD:1/4" UNLESS OTHERWISE NOTED. CLOSED WITH 135° BENDS. 9. PROVIDE MEASURES NECESSARY TO PROTECT THE STRUCTURE DURING CONSTRUCTION, SUCH MEASURES INCLUDE. IN WALL ELEMENTS, VERTICAL BARS SHALL BE LOCATED ON OUTERMOST LAYER UNLESS SPECIFICALLY NOTED BUT MAY NOT BE LIMITED TO, BRACING AND SHORING FOR LOADS DURING CONSTRUCTION, RETAIN A REGISTERED CIV. ENGINEER WHOM IS PROPERLY QUALIFIED TO DESIGN BRACING, SHORING, ETC. VISITS TO THE SITE BY THE OWNER'S 21. ALL STEEL FABRICATION AND DETAILS TO COMPLY WITH MOST STRINGENT OF: AISC CODE, AWS CODE, AND THE 2019 OTHERWISE REPRESENTATIVE WILL NOT INCLUDE OBSERVATION OF THE ABOVE NOTED ITEMS. ALL WELDING TO BE BY AWS CERTIFIED WELDERS AND SHALL CONFORM TO ALL 2019 CBC AND AWS REQUIREMENTS. 10. INFORMATION SHOWN ON THE DRAWINGS RELATED TO EXISTING CONDITIONS REPRESENTS THE PRESENT KNOWLEDGE, ALL WELDERS SHALL BE PRE-QUALIFIED BY THE PROJECT WELDING INSPECTOR FOR THE WELD TYPES AND POSITIONS USED IN THE PROCEDURES THEY WILL BE PERFORMING. BUT WITHOUT GUARANTEE OF ACCURACY. REPORT CONDITIONS THAT CONFLICT WITH THE CONTRACT DOCUMENTS TO THE OWNER'S REPRESENTATIVE. DO NOT DEVIATE FROM THE CONTRACT DOCUMENTS WITHOUT WRITTEN DIRECTION 23. UNLESS NOTED OTHERWISE, ALL STEEL EXPOSED TO WEATHER SHALL BE HOT DIP GALVANIZED, UNLESS IT IS PART OF THE PAINTED TANK ASSEMBLY. 11. THE CONTRACTOR IS SOLELY RESPONSIBLE FOR PROVIDING A SAFE PLACE TO WORK AND MEETING THE REQUIREMENTS OF ALL APPLICABLE JURISDICTIONS. EXECUTE WORK TO ENSURE THE SAFETY OF PERSONS AND ADJACENT PROPERTY AGAINST DAMAGE BY FALLING DEBRIS AND OTHER HAZARDS IN CONNECTION WITH THIS WORK HELICAL ANCHORS DEFERRED SUBMITTALS 12. UNLESS NOTED OTHERWISE, REFER TO DRAWINGS OTHER THAN STRUCTURAL FOR FINISHES, SLOPES, DEPRESSIONS. OPENINGS, CURBS, STAIRS, RAMPS, TRENCHES, EQUIPMENT AND LOCATIONS AND EXTENT OF SUCH CONDITIONS CONTRACTOR SHALL SUBMIT HELICAL ANCHOR SHOP DRAWINGS INCLUDING MANUFACTURER DATA. CURRENT VALID DEFERRED SUBMITTALS ARE BY THE CONTRACTOR. DETAILS SHOWN IN THESE SECTIONS ARE FOR BIDDING 13. CONTRACTOR TO COORDINATE ALL NEW WORK WITH EXISTING SITE CONDITIONS AND REPORT ANY DISCREPANCIES TO ENGINEERING TEST REPORTS, AND SITE SPECIFIC CAPACITY CALCULATIONS STAMPED AND SIGNED BY A CALIFORNIA LICENSED. PURPOSES ONLY AND NOT FOR CONSTRUCTION. THE ENGINEER PRIOR TO CONSTRUCTION PROFESSIONAL ENGINEER. SITE SPECIFIC ANCHOR CALCULATIONS SHALL BE BASED UP ON THE PROJECT GEOTECHNICAL DEFERRED SUBMITTALS INCLUDE: 14. DETAILS OR CONDITIONS NOT FULLY DEVELOPED ON STRUCTURAL DOCUMENTS ARE SIMILAR TO DEVELOPED DETAILS. 1 MG SAMOA TANK ROOF REPLACEMENT FOR BIDDING PURPOSES, ANCHOR SCHEMATIC DESIGN IS BASED UPON HUBBELL POWER SYSTEMS INC. CHANCE ANCHORS PER ESR-2794. 1 MG SAMOA TANK LADDERS AND PLATFORM 15. SEE SPECIFICATIONS FOR ADDITIONAL REQUIREMENTS. ANCHOR DESIGN SHALL PROVIDE FOR A MINIMUM 50 YEAR DESIGN LIFE WITH CORROSION ALLOWANCE BASED UPON THE SITE CONTRACTOR SHALL SUBMIT STEEL TANK ROOF PLANS AND CALCULATIONS TO THE OWNER FOR APPROVAL AND PERMIT PRIOR TO ANY CONSTRUCTION. PLANS AND CALCULATIONS MUST BE PREPARED, SEALED, AND SIGNED BY A SPECIFIC SOIL CORROSIVITY ANALYSIS REPORT. 16. ALL PLANS TO BE COORDINATED WITH GENERAL NOTES AND TYPICAL DETAILS AS APPLICABLE. CALIFORNIA LICENSED ENGINEER. CALCULATIONS SHALL INCLUDE ANALYSIS OF NEW ROOF WITH SEISMIC SLOSHING MINIMUM ANCHOR CAPACITY AND EMBEDMENT PARAMETERS ARE AS FOLLOWS 17. ALL LADDERS, RAILINGS, PLATFORMS, AND SAFETY ELEMENTS SHALL BE PROVIDED IN CONFORMANCE WITH CAL / OSHA a. 1MG SAMOA TANK, 13 KIP TENSION CAPACITY WITH ANCHORS AT 4'-0" O/C AROUND PERIMETER. FINAL EMBEDMENT LENGTH CONTRACTOR'S STEEL TANK ROOF SUBMITTAL PACKAGE WILL BE SUBJECT TO OWNER REVIEW AND COMMENT. BY ANCHOR DESIGNER, APPROXIMATED AT 17 FEET MAXIMUM AS TO REMAIN ABOVE EXISTING GROUND WATER TABLE AS CONTRACTOR WILL BE RESPONSIBLE FOR ADDRESSING OWNER REVIEW COMMENTS AND RESUBMITTING THE TANK ROOF SUBMITTAL AS NECESSARY. OWNER RESERVES RIGHT TO REQUEST DESIGN MODIFICATIONS BASED ON SERVICEABILITY / MAINTENANCE REQUIREMENTS, ETC. IDENTIFIED IN THE PROJECT GEOTECHNICAL REPORT. **FOUNDATIONS** SPECIAL INSPECTIONS FOLINDATION DESIGN WILL BE BASED ON CRITERIA AND RECOMMENDATIONS PRESENTED IN THE GEOTECHINCAL SPECIAL INSPECTION IN ACCORDANCE WITH 2019 CALIFORNIA BUILDING CODE CHAPTER 17 IS REQUIRED ON THE INVESTIGATION REPORT: HBMWD RESERVOIRS SEISMIC RETROFIT PROJECT, THREE WATER TANKS, KORBLEX AND FOLLOWING PORTIONS OF THE WORK: SAMOA, CALIFORNIA, PREPARED BY PREPARED BY CRAWFORD & ASSOCIATES, INC. DATED JULY 2021 STRUCTURAL STEEL CONCRETE ALLOWABLE BEARING PRESSURE FOR TANK FOUNDATIONS IS 3,000 PSF WITH A 1/3 INCREASE FOR SEISMIC, FOR BOTH HELICAL ANCHORS 2. (REFER TO THE STATEMENT OF SPECIAL INSPECTIONS FOR MORE SPECIFIC REQUIREMENTS) Client HUMBOLDT BAY MUNICIPAL WATER ** STRUCTURAL GENERAL NOTES DISTRICT original size sheet GHD 718 Third Street 0 Eureka California 95501 USA * SAMOA RESERVOIR SEISMIC T 1 707 443 8326 F 1 707 444 8330 0 ISSUE FOR BID NS NS 2024-05-01 RETROFIT Checked Approved No. Issue Date Conditions of Use Author S. GOULD This document and the ideas and designs incorporated herein, as an instrument of professional service, is the property of GHD. This document may only be used by GHD's client (and any other person who GHD has agreed can use this document (for the purpose of which it was prepared and must not be used by nyther person or for ny other purpose.) 12627733 Drafting Check S. MCHANEY Project Manager N. STEVENS 2024-05-01 **AS SHOWN** Design Check B. CROWELL Project Director K. TOBIN Designer S. GOULD

STATEMENT OF SPECIAL INSPECTIONS

TITEM 4. MATERIAL MEDICIOATION OF HIGH CERENOTH

STATEMENT OF SPECIAL INSPECTIONS

THIS STATEMENT OF SPECIAL INSPECTIONS IS SUBMITTED IN ACCORDANCE WITH THE SPECIAL INSPECTION AND STRUCTURAL TESTING REQUIREMENTS OF THE BUILDING CODE SECTIONS 1704 AND 1705

THIS STATEMENT OF SPECIAL INSPECTIONS ENCOMPASS THE FOLLOWING DISCIPLINES:

STRUCTURAL SPECIAL INSPECTIONS PER 1704

STRUCTURAL SPECIAL INSPECTIONS FOR SEISMIC RESISTANCE ☐ STRUCTURAL SPECIAL INSPECTIONS FOR WIND RESISTANCE

THE SCHEDULE OF SPECIAL INSPECTIONS SUMMARIZES THE SPECIAL INSPECTIONS AND TESTS REQUIRED. SPECIAL INSPECTORS WILL REFER TO THE APPROVED PLANS AND SPECIFICATIONS FOR DETAILED SPECIAL INSPECTION REQUIREMENTS. ANY ADDITIONAL TESTS AND INSPECTIONS REQUIRED BY THE APPROVED PLANS AND SPECIFICATIONS WILL ALSO BE PERFORMED.

THE SPECIAL INSPECTIONS IDENTIFIED ARE IN ADDITION TO THOSE REQUIRED BY OTHER SECTIONS OF THE

THE SPECIAL INSPECTION COORDINATOR SHALL KEEP RECORDS OF ALL INSPECTIONS AND SHALL FURNISH INSPECTION REPORTS TO THE OWNER AND THE REGISTERED DESIGN PROFESSIONAL IN RESPONSIBLE CHARGE. DISCOVERED DISCREPANCIES SHALL BE BROUGHT TO THE IMMEDIATE ATTENTION OF THE CONTRACTOR FOR CORRECTION. IF SUCH DISCREPANCIES ARE NOT CORRECTED, THE DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE OWNER AND THE REGISTERED DESIGN PROFESSIONAL IN RESPONSIBLE CHARGE. THE SPECIAL INSPECTION PROGRAM DOES NOT RELIEVE THE CONTRACTOR OF

INTERIM REPORTS SHALL BE SUBMITTED TO THE OWNER AND THE REGISTERED DESIGN PROFESSIONAL IN RESPONSIBLE CHARGE IN ACCORDANCE WITH SECTION 1704.1.2.

A FINAL REPORT OF SPECIAL INSPECTIONS DOCUMENTING COMPLETION OF ALL REQUIRED SPECIAL INSPECTIONS, TESTING AND CORRECTION OF ANY DISCREPANCIES NOTED IN THE INSPECTIONS SHALL BE SUBMITTED PRIOR TO PROJECT COMPLETION. THE FINAL REPORT WILL DOCUMENT THE REQUIRED SPECIAL INSPECTIONS AND CORRECTION OF DISCREPANCIES NOTED IN INSPECTIONS.

JOB SITE SAFETY AND MEANS AND METHODS OF CONSTRUCTION ARE SOLELY THE RESPONSIBILITY OF THE

THE CONTRACTOR IS REQUIRED TO COORDINATE ALL INSPECTIONS. THE CONTRACTOR SHALL NOTIFY THE OWNER'S REPRESENTATIVE AND THE SPECIAL INSPECTOR A MINIMUM OF 24 HOURS PRIOR TO ANY OF EMPIRICATIONS THAT ARE REQUIRED. THE CONTRACTOR SHALL NOTIFY THE OWNER'S REPRESENTATIVE AND THE SPECIAL INSPECTOR A MINIMUM OF 24 HOURS PRIOR TO ANY CONCRETE TO BE POURED. SPECIAL INSPECTIONS THAT ARE REQUIRED. THE CONTRACTOR SHALL NOTIFY THE OWNER'S

ALL SPECIAL INSPECTORS AND QUALIFICATIONS SHALL BE SUBMITTED TO THE ENGINEER-OF-RECORD AND

SPECIALLY INSPECTED WORK THAT IS INSTALLED OR COVERED WITHOUT THE APPROVAL OF THE OWNER

CONTINUOUS INSPECTION IS ALWAYS REQUIRED DURING THE PERFORMANCE OF THE WORK UNLESS OTHERWISE SPECIFIED. WHEN WORK IN MORE THAN ONE CATEGORY OF WORK REQUIRING SPECIAL INSPECTION IS TO BE PERFORMED SIMULTANEOUSLY, OR THE GEOGRAPHIC LOCATION OF THE WORK IS SUCH THAT IT CANNOT BE CONTINUOUSLY OBSERVED, IT IS THE AGENT'S RESPONSIBILITY TO EMPLOY A SUFFICIENT NUMBER OF INSPECTORS TO ASSURE THAT ALL THE WORK IS INSPECTED IN ACCORDANCE WITH THE PROVISIONS OF THE BUILDING CODE.

CONTRACTOR STATEMENT OF RESPONSIBILITY

COMPONENT DESIGNATED ABOVE AS PART OF THE MAIN WIND FORCE OR MAIN SEISMIC FORCE RESISTING SYSTEMS ABOVE MUST SUBMIT A STATEMENT OF RESPONSIBILITY PER SECTION 1706.

SCHEDULE OF INSPECTION AND TESTING AGENCIES

THIS STATEMENT OF SPECIAL INSPECTIONS / QUALITY ASSURANCE PLAN INCLUDES THE FOLLOWING BUILDING SYSTEMS:

SOILS AND FOUNDATIONS ☐ CAST-IN-PLACE CONCRETE
 ☐ PRECAST CONCRETE

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☐ WOOD CONSTRUCTION ☐ MECHANICAL & ELECTRICAL SYSTEMS ☐ ARCHITECTURAL SYSTEMS

STRUCTURAL STEEL STRUCTURAL STEEL
COLD-FORMED STEEL FRAMING MASONRY LEVEL 2

SPECIAL INSPECTION AGENCIES	FIRM AND CONTACT INFO.
SPECIAL INSPECTION COORDINATOR	TBD
2. CONCRETE INSPECTOR	TBD
3. STEEL INSPECTOR	TBD
4. SOILS INSPECTOR	TBD
5. CONCRETE TESTING AGENCY	TBD

QUALIFICATIONS OF INSPECTORS AND TESTING TECHNICIANS

THE QUALIFICATIONS OF ALL PERSONNEL PERFORMING SPECIAL INSPECTION AND TESTING ACTIVITIES ARE SUBJECT TO THE APPROVAL OF OWNER. THE CREDENTIALS OF ALL INSPECTORS AND TESTING TECHNICIANS SHALL BE PROVIDED IF REQUESTED.

KEY FOR MINIMUM QUALIFICATIONS OF INSPECTION AGENTS:

WHEN THE REGISTERED DESIGN PROFESSIONAL IN RESPONSIBLE CHARGE DEEMS IT APPROPRIATE THAT THE INDIVIDUAL PERFORMING A STIPULATED TEST OR INSPECTION HAVE A SPECIFIC CERTIFICATION OR LICENSE AS INDICATED BELOW, SUCH DESIGNATION SHALL APPEAR BELOW THE AGENCY NUMBER ON THE SCHEDULE.

STRUCTURAL ENGINEER - A LICENSED SE OR PE SPECIALIZING IN THE DESIGN OF BUILDING STRUCTURES GEOTECHNICAL ENGINEER - A LICENSED GE OR PE SPECIALIZING IN SOIL MECHANICS AND FOUNDATIONS ENGINEER-IN-TRAINING - A GRADUATE ENGINEER WHO HAS PASSED THE FUNDAMENTALS OF ENGINEERING

AMERICAN CONCRETE INSTITUTE (ACI) CERTIFICATION

CONCRETE FIELD TESTING TECHNICIAN - GRADE 1 CONCRETE CONSTRUCTION INSPECTOR
LABORATORY TESTING TECHNICIAN - GRADE 1&2 STRENGTH TESTING TECHNICIAN

AMERICAN WELDING SOCIETY (AWS) CERTIFICATION

AWS-CWI CERTIFIED WELDING INSPECTOR
AWS/AISC-SSI CERTIFIED STRUCTURAL STEEL INSPECTOR

INTERNATIONAL CODE COUNCIL (ICC) CERTIFICATION

STRUCTURAL MASONRY SPECIAL INSPECTOR STRUCTURAL STEEL AND WELDING SPECIAL INSPECTOR SPRAY-APPLIED FIREPROOFING SPECIAL INSPECTOR PRESTRESSED CONCRETE SPECIAL INSPECTOR ICC-RCSI REINFORCED CONCRETE SPECIAL INSPECTOR

AMERICAN SOCIETY OF NONDESTRUCTIVE TESTING (ASNT)

TABLE 1705.6 - SOILS VERIFY MATERIALS BELOW SHALLOW FOUNDATIONS ARE AGENCY # (QUALIF.): PE/GE ADEQUATE TO ACHIEVE THE DESIGN BEARING CAPACITY. ▼ PERIODIC ☐ CONTINUOUS VERIFY EXCAVATIONS ARE EXTENDED TO PROPER DEPTH AGENCY # (QUALIF.): PE/GE AND HAVE REACHED PROPER MATERIAL. ☑ PERIODIC ☐ CONTINUOUS PERFORM CLASSIFICATION AND TESTING OF COMPACTED AGENCY # (QUALIF.): PE/GE ITEM 3: ☑ PERIODIC ☐ CONTINUOUS VERIFY USE OF PROPER MATERIALS, DENSITIES AND LIFT AGENCY # (QUALIF.): PE/GE THICKNESSES DURING PLACEMENT AND COMPACTION OF □ PERIODIC CONTINUOUS PRIOR TO PLACEMENT OF COMPACTED FILL, INSPECT SUBGRADE AND VERIFY THAT SITE HAS BEEN PREPARED ITEM 5: AGENCY # (QUALIF.): PE/GE PROPERLY. ☑ PERIODIC ☐ CONTINUOUS

TABLE 1705.2 - STEEL CONSTRUCTION

ITEM 1:	MATERIAL VERIFICATION OF HIGH-STRENGTH BOLTS, NUTS, AND WASHERS.	AGENCY # (QUALIF.): AWS/AISC-SSI, ICC-SWSI
	ITIFICATION MARKINGS TO CONFORM TO ASTM STANDARDS CIFIED IN THE APPROVED CONSTRUCTION DOCUMENTS. PERIODIC CONTINUOUS	REFERENCE STDS.: AISC 360, SECTION A3.3 AND APPLICABLE ASTM MATERIAL STANDARDS
B. MAN	UFACTURER'S CERTIFICATE OF COMPLIANCE REQUIRED. ☑ PERIODIC ☐ CONTINUOUS	
ITEM 2:	INSPECTION OF HIGH-STRENGTH BOLTING:	AGENCY # (QUALIF.): AWS/AISC-SSI, ICC-SWSI
SCOPE: A. SNU	IG-TIGHT JOINTS. ☑ PERIODIC ☐ CONTINUOUS	
TUR	TENSIONED AND SLIP-CRITICAL JOINTS USING N-OF-NUT MATCHMAKING, TWIST-OFF BOLT OR DIRECT SION INDICATOR METHODS OF INSTALLATION PRICE PERIODIC CONTINUOUS	REFERENCE STDS. AISC 360 SECTION M2.5
C. PRE Tur Wre	TENSIONED AND SLIP-CRITICAL JOINTS USING N-OF-NUT WITHOUT MATCHMARKING OR CALIBRATED NICH METHODS OF INSTALLATION. PERIODIC 🔯 CONTINUOUS	
ITEM 3:	MATERIAL VERIFICATION OF STRUCTURAL STEEL	AGENCY # (QUALIF.): PE/SE
	STRUCTURAL STEEL, IDENTIFICATION MARKINGS TO IFORM TO AISC 360 IN PERIODIC CONTINUOUS	REFERENCE STDS. AISC 360, SECTION A3.3 AND APPLICABLE ASTM MATERIAL STANDARDS
TO	OTHER STEEL, IDENTIFICATION ON MARKINGS TO CONFORM ASTM STANDARDS SPECIFIED IN APPROVED CONSTRUCTION UMENTS. IN PERIODIC CONTINUOUS	REFERENCE STDS. APPLICABLE ASTM MATERIAL STANDARDS
C. MAN	UFACTURER'S CERTIFIED TEST REPORTS ☑ PERIODIC ☐ CONTINUOUS	
ITEM 4:	MATERIAL VERIFICATION OF COLD-FORMED STEEL DECK	AGENCY # (QUALIF.): AWS-CWI, ASNT
SCOPE: A. MAN	UFACTURER'S CERTIFIED TEST REPORTS ☑ PERIODIC □ CONTINUOUS	
ITEM 5:	MATERIAL VERIFICATION OF WELD FILLER MATERIALS	AGENCY # (QUALIF.): AWS-CWI, ASNT
	ATTIFICATION MARKINGS TO CONFORM TO AWS SPECIFICATION THE APPROVED CONSTRUCTION DOCUMENTS. ME PERIODIC CONTINUOUS	REFERENCE STDS. AISC 360, SECTION A3.5 AND APPLICABLE AWS A5 DOCUMENTS
B. MAN	IUFACTURER'S CERTIFICATE OF COMPLIANCE REQUIRED. ☑ PERIODIC ☐ CONTINUOUS	
ITEM 6:	INSPECTION OF WELDING	AGENCY # (QUALIF.): AWS-CWI, ASNT
SCOPE: A. STR	UCTURAL STEEL AND COLD-FORMED STEEL DECK	
1.	COMPLETE AND PARTIAL JOINT PENETRATION GROOVE WELDS	
2.	☐ PERIODIC ☑ CONTINUOUS MULTIPASS FILLET WELDS ☐ PERIODIC ☑ CONTINUOUS	
	SINGLE-PASS FILLET WELDS > 1/6".	REFERENCE STDS. AWS D1.1
3.	☐ PERIODIC ☑ CONTINUOUS	Allo Dill
4.	PLUG AND SLOT WELDS ☐ PERIODIC ☑ CONTINUOUS	Ans Util
4. 5.	PLUG AND SLOT WELDS ☐ PERIODIC ☑ CONTINUOUS SINGLE—PASS FILLET WELDS < 5/6". ☑ PERIODIC ☐ CONTINUOUS	REFERENCE STDS.
5.	PLUG AND SLOT WELDS □ PERIODIC SINGLE-PASS FILLET WELDS < %6". ■ PERIODIC □ CONTINUOUS FLOOR AND ROOF DECK WELDS. ■ PERIODIC □ CONTINUOUS	
4. 5. 6. B. REIN	PLUG AND SLOT WELDS □ PERIODIC	REFERENCE STDS.
5.	PLUG AND SLOT WELDS □ PERIODIC SINGLE-PASS FILLET WELDS < %6". ■ PERIODIC □ CONTINUOUS FLOOR AND ROOF DECK WELDS. ■ PERIODIC □ CONTINUOUS	REFERENCE STDS.
4. 5. 6. B. REIN	PLUG AND SLOT WELDS PERIODIC SINGLE-PASS FILLET WELDS < %/6". PERIODIC CONTINUOUS FLOOR AND ROOF DECK WELDS. PERIODIC CONTINUOUS FLOOR STEEL: VERIFICATION OF WELDABILITY OF REINFORCING STEEL OTHER THAN ASTM A706	REFERENCE STDS.
4. 5. 6. B. REIN 1.	PLUG AND SLOT WELDS PERIODIC	REFERENCE STDS. AWS D1.3 REFERENCE STDS. AWS D1.4

ITEM 7: INSPECTION OF STEEL FRAME JOINT DETAILS FOR COMPLIANCE	AGENCY # (QUALIF.): AWS-CWI, ASNT
SCOPE: A. DETAILS SUCH AS BRACING AND STIFFENING. ME PERIODIC CONTINUOUS	
B. MANUFACTURER'S CERTIFICATE OF COMPLIANCE REQUIRED. ☑ PERIODIC ☐ CONTINUOUS	
C. MANUFACTURER'S CERTIFICATE OF COMPLIANCE REQUIRED. ☑ PERIODIC ☐ CONTINUOUS	

TARLE 1705 3 - CONCRETE CONSTRUCTION

TABLE 1705.3 - CONCRETE CONSTRUCTION							
TIEM 1: INSPECT REINFORCEMENT, INCLUDING PRESTRESSING TENDONS, AND VERIFY PLACEMENT. ☑ PERIODIC ☐ CONTINUOUS	AGENCY # (QUALIF.): ACI-CCI, ICC-RCSI REFERENCE STDS: ACI 318: CH. 20, 25.2, 25.3, 26.6.1-26.6.3						
ITEM 2: REINFORCING BAR WELDING:	AGENCY # (QUALIF.): ACI-CWI						
SCOPE: A. VERIFY WELDABILITY OF REINFORCING BARS OTHER THAN ASTM A706;							
□ PERIODIC □ CONTINUOUS	REFERENCE STDS.: AWS D1.4						
B. INSPECT SINGLE—PASS FILLET WELDS, MAXIMUM ¾6"; AND. ☑ PERIODIC ☐ CONTINUOUS	ACI 318: 26.6.4						
C. INSPECT ALL WELDS. □ PERIODIC □ CONTINUOUS							
ITEM 3: INSPECT ANCHORS CAST IN CONCRETE:	AGENCY # (QUALIF.): ACI-CCI, ICC-RCSI						
■ PERIODIC □ CONTINUOUS	REFERENCE STDS: ÁCI 318 17.8.2						
ITEM 4: INSPECT TEST ANCHORS POST-INSTALLED IN HARDENED CONCRETE MEMBERS. SCOPE:	AGENCY # (QUALIF.): ACI-CCI, ICC-RCSI						
A. ADHESIVE ANCHORS INSTALLED IN HORIZONTALLY OR UPWARDLY INCLINED ORIENTATIONS TO RESIST SUSTAINED TENSION LOADS. □ PERIODIC ☑ CONTINUOUS	REFERENCE STDS.: ACI 318: 17.8.2.4						
B. MECHANICAL ANCHORS AND ADHESIVE ANCHORS NOT DEFINED IN 4.A.	REFERENCE STDS.: ACI 318: 17.8.2						
✓ PERIODIC							
ITEM 5: VERIFY USE OF REQUIRED DESIGN MIX: ☑ PERIODIC ☐ CONTINUOUS	AGENCY # (QUALIF.): ACI-CCI, ICC-RCSI ACI 318 CH. 19, 26.4.3, 26.4.4						
TITEM 6: PRIOR TO CONCRETE PLACEMENT, FABRICATED SPECIMENS FOR STRENGTH TESTS, PERFORM SLUMP AND AIR CONTENT TESTS, AND DETERMINE THE TEMPERATURE OF THE CONCRETE. ☐ PRIODIC ▼ CONTINUOUS	AGENCY # (QUALIF.): ACI-CFTT, ACI-STT REFERENCE STDS.: ASTM C172 ASTM C31 ACI 318: 26.4, 26.12						
ITEM 7: INSPECT CONCRETE AND SHOTCRETE PLACEMENT FOR							
PROPER APPLICATION TECHNIQUES. ☐ PERIODIC ☑ CONTINUOUS	AGENCY # (QUALIF.): ACI-CCI, ICC-RCSI REFERENCE STDS.: ACI 318: 26.5, ACI 506: 3.4						
ITEM 8: VERIFY MAINTENANCE OF SPECIFIED CURING TEMPERATURE AND TECHNIQUES.	AGENCY # (QUALIF.): ACI-CCI, ICC-RCSI REFERENCE STDS.:						
■ PERIODIC	ACI 318: 26.5.3-26.5.5						
ITEM 9: INSPECT PRESTRESSED CONCRETE FOR:	AGENCY # (QUALIF.): ACI-CCI, ICC-RCSI REFERENCE STDS.: ACI 318: 26.10.2						
A. APPLICATION OF PRESTRESSING FORCES; AND.							
PERIODIC CONTINUOUS							
B. GROUTING OF BONDED PRESSTRESSING TENDONS. ☐ PERIODIC ☑ CONTINUOUS							
ITEM 10: INSPECT ERECTION OF PRECAST CONCRETE MEMBERS.	AGENCY # (QUALIF): ACI-CCI ICC-RCSI						
■ PERIODIC	AGENCY # (QUALIF.): ACI-CCI, ICC-RCSI REFERENCE STDS.: ACI 318: 26.9.2						
TITEM 11: VERIFY IN-SITU CONCRETE STRENGTH, PRIOR TO STRESSING OF TENDONS IN POST-TENSIONED CONCRETE AND PRIOR TO REMOVAL OF SHORES AND FORMS WHEN BEAMS AND STRUCTURAL SLABS. ■ PERIODIC □ CONTINUOUS	AGENCY # (QUALIF.): ACI-CFTT, ACI-STT REFERENCE STDS.: ACI 318: 26.10.2, 26.11.2						
ITEM 11: INSPECT FORMWORK FOR SHAPE, LOCATION AND	AGENCY # (QUALIF.): ACI-CCI, ICC-RCSI						
DIMENSIONS OF THE CONCRETE MEMBER BEING FORMED. IN PERIODIC CONTINUOUS	REFERENCE STDS.: ACI 318: 26.11.2(B)						

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Designer S GOULD	Design Check B CROWELL	Project Director K TORIN



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A	DISTRICT
	Project SAMOA RESERVOIR SEISMIC
	RETROFIT

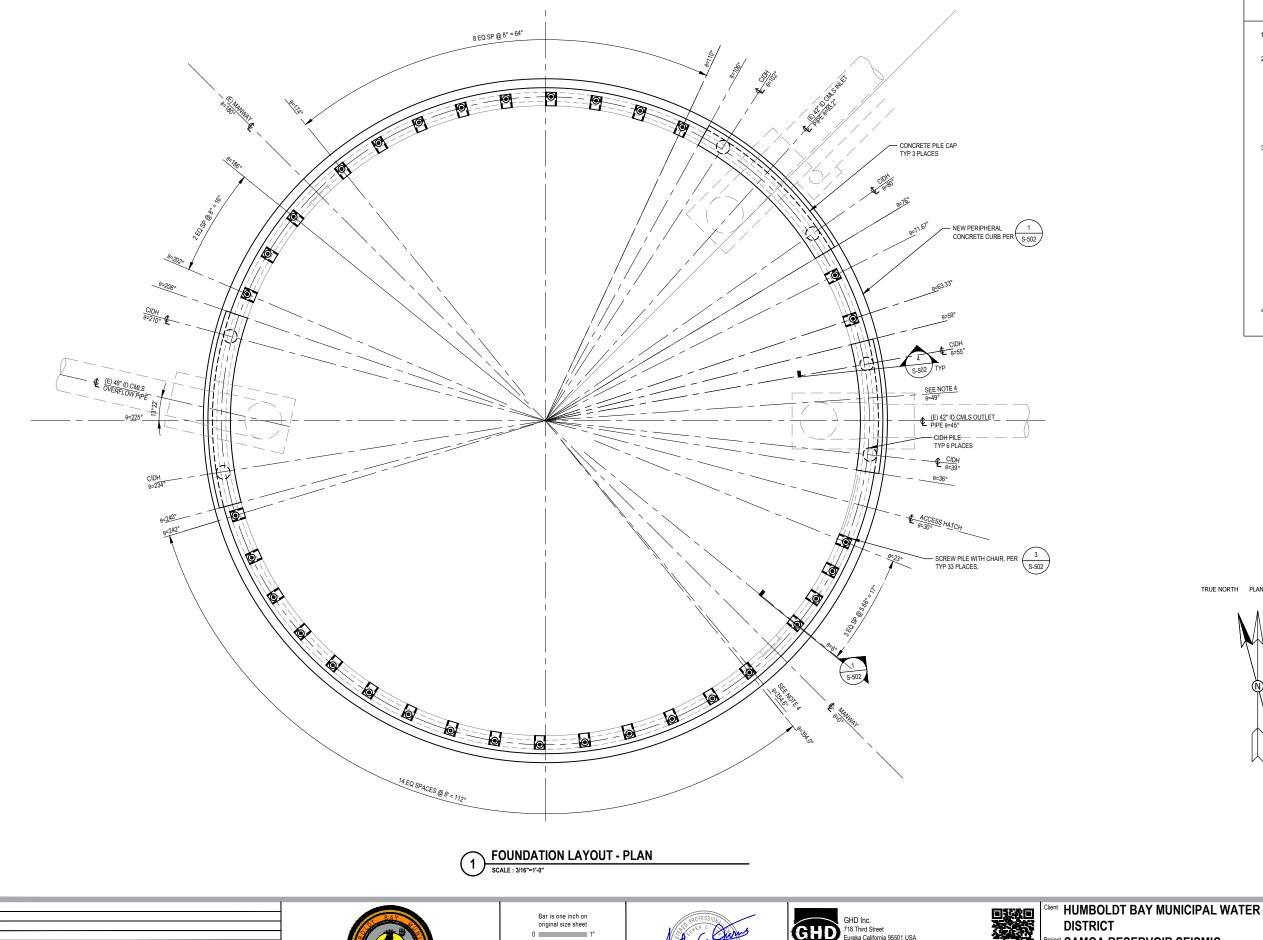
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AS SHOWN

** SPECIAL INSPECTIONS

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Plotted By: Michelle Davidson



SHEET GENERAL NOTES

- CONTRACTOR TO PROVIDE ALL COMPONENTS TO CONSTRUCT / INSTALL NEW WORK.
- ALL ANGULAR MEASUREMENTS SHOWN ARE APPROXIMATE AND SHALL BE ALL ANGULAR MEASUREMENTS SHOWN ARE APPROXIMATE AND SHALL BE VERIFIED IN THE FIELD PRIOR TO ANY CONSTRUCTION OR FABRICATION. ANY DISCREPANCIES BETWEEN THIS LAYOUT PLAN AND FIELD MEASUREMENTS MADE BY THE CONTRACTOR SHALL BE SUBMITTED TO THE ENGINEER OF RECORD PRIOR TO ANY CONSTRUCTION OR FABRICATION. THE ENGINEER OF RECORD MAY MODIFY THIS LAYOUT TO MITIGATE ANY DISCREPANCIES AND ISSUE AN ADDENDUM TO THIS DRAWING. NO CONSTRUCTION OR FABRICATION SHALL BEGIN PRIOR TO RECEIPT OF THE ADDENDUM OR APPROVAL FROM THE ENGINEER OF PRECORD.
- 3. VERTICAL HELICAL SCREW ANCHOR (DESIGN AND CONSTRUCTION BY CONTRACTOR REFERENCE SPECIFICATION SECTION 31 62 99). CHANCE/HUBBLE COMBINATION HELICAL PILE OR APPROVED EQUIVALENT, PILE SYSTEM TO BE MANUFACTURED AND INSTALLED PER THE FOLLOWING CRITERIA:
 SHAFT MATERIAL:
 TYPE SS200 2.0"x2.0" SOLID SQUARE SHAFT

EXTENSION: TYPE RS3500.300 (3.5 O.D., 0.300" WALL PIPE SHAFT)
HELICAL CONFIGURATION:
8"x10"x12" HELIX
ESTIMATED LENGTH:

22-25 FT (UPPERMOST HELIX TO BE LOCATED 5-FT BELOW FINISHED GRADE OR LIQUIFIABLE LAYER)
REQUIRED CAPACITY:

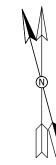
REQUIRED CAPACITY:
COMPRESSION: 70-KIPS ULTIMATE
TENSION: 60-KIPS ULTIMATE
MAX INSTALLATION TORQUE: 13,000 FT-LBS

(RS3500,300 SHAFT MAX WORKING TORQUE LIMIT)

MAX INSTALLATION TORQUE: 10,000 FT-LBS

REMOVE PIPE PENETRATION FLUSH WITH EXTERIOR SHELL. PROVIDE $\mspace{1}{2}"$ THICK x 4" SQUARE PLATE AT INTERIOR WITH $\mspace{1}{3}"$ FILLET WELD ALL AROUND.

TRUE NORTH PLAN NORTH



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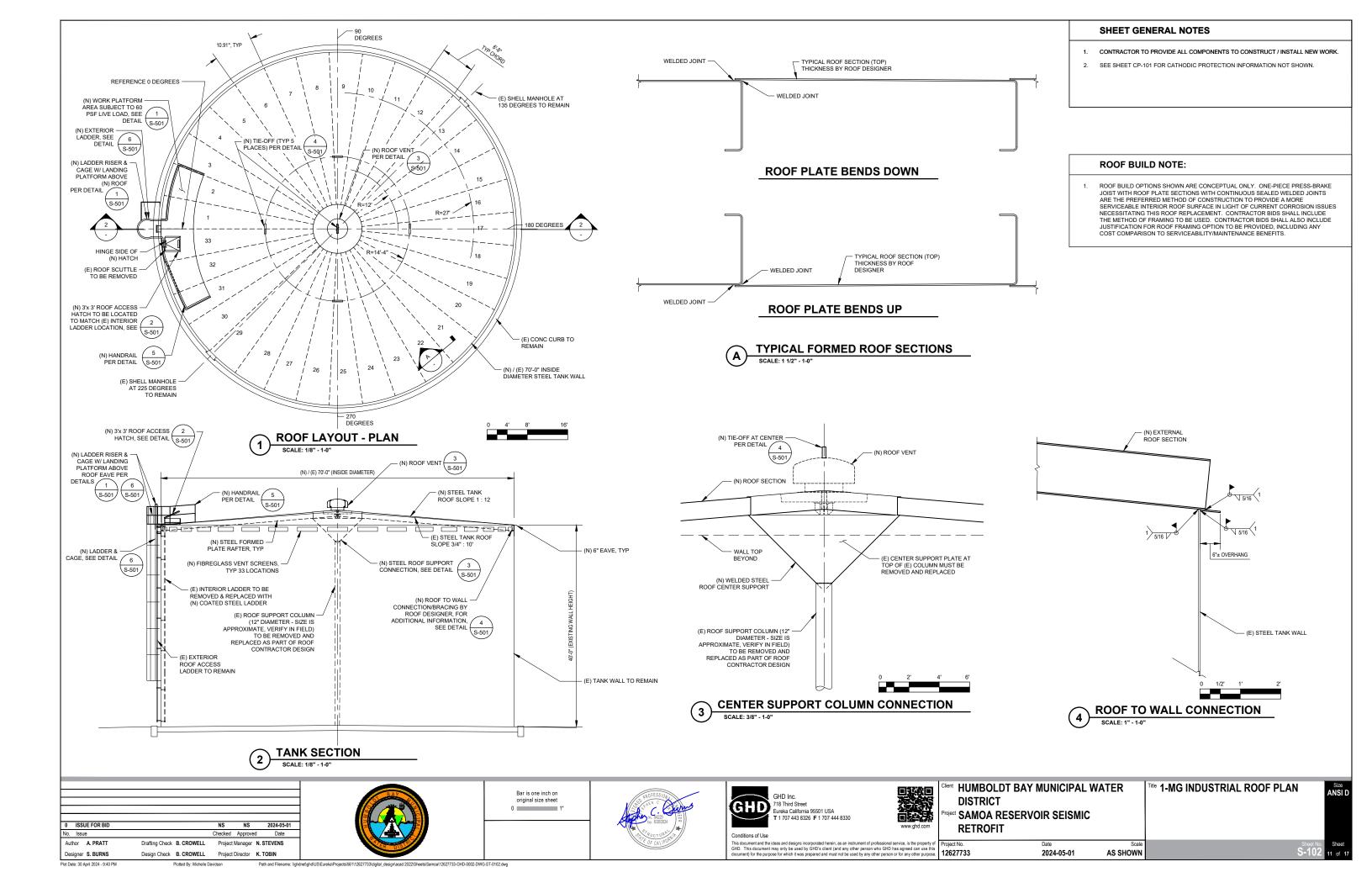
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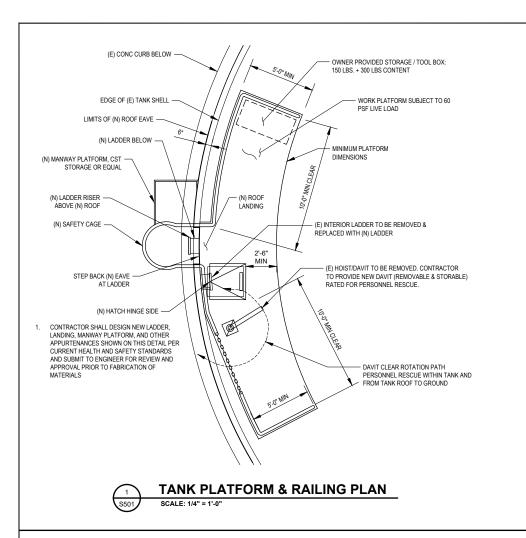
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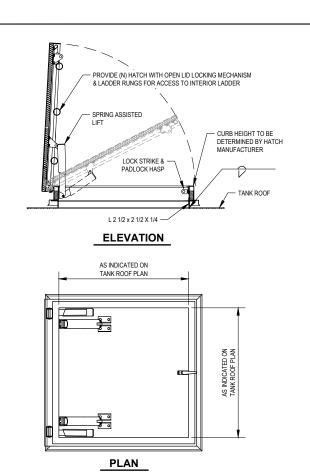
12627733 2024-05-01 ite 1-MG INDUSTRIAL TANK **FOUNDATION PLAN**

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Plot Date: 30 April 2024 - 9:39 PM

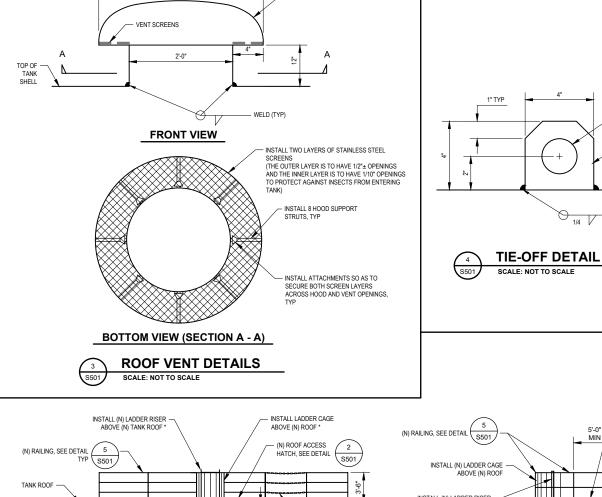




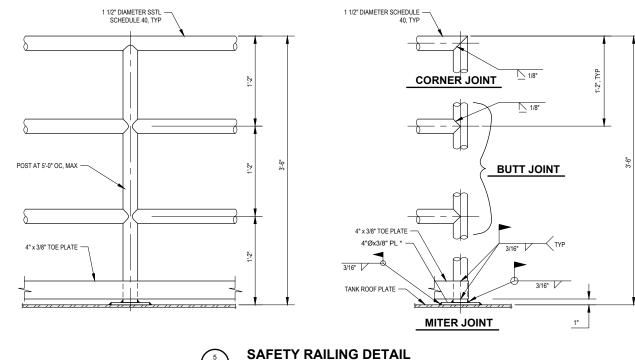


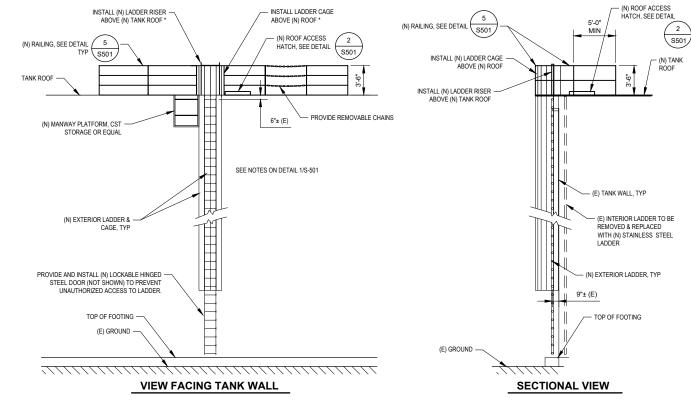
ROOF HATCH DETAIL

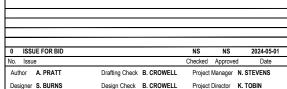
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- ALUMINUM OR SSTL HOOD







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GHD Inc. 718 Trird Street Eureka California 95501 USA T 1707 443 8326 F 1707 444 8330 Conditions of Use	Client HUMBOLDT BAY MUNICIPAL WATER DISTRICT Project SAMOA RESERVOIR SEISMIC RETROFIT
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SCALE: NOT TO SCALE

EXTERIOR LADDER DETAILS

AS SHOWN

TYPICAL DETAILS - 1

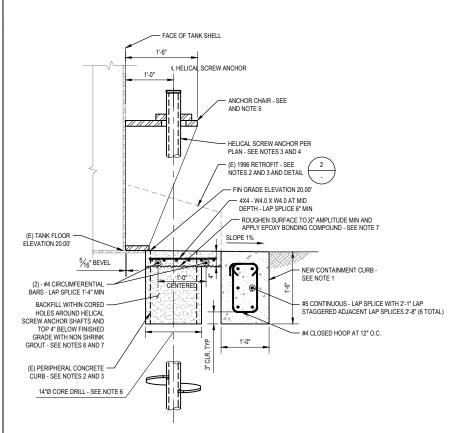
~ 2" DIAMETER

1/2" THICK

- WELD (TYP)

TIE-OFF LUG TO BE

- TOP OF TANK SHELL

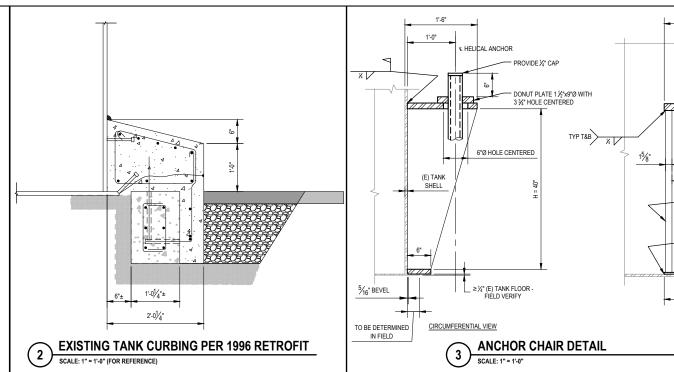


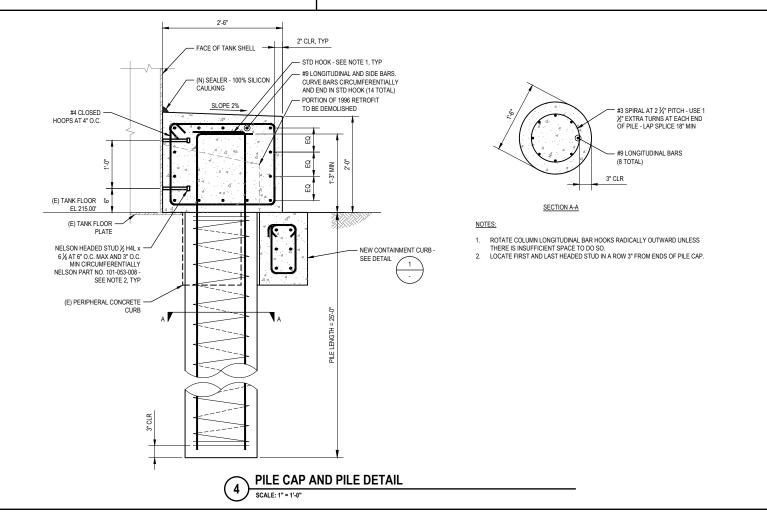
CONSTRUCTION SEQUENCING:

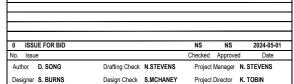
- INSTALL NEW CONTAINMENT RING PRIOR TO ANY OTHER DEMOLITION. CONSTRUCTION, OR HELICAL PILE INSTALLATION. DO NOT PERFORM SEQUENCE NOTES 3 THROUGH 7 UNTIL NEW CONTAINMENT RING HAS ACHIEVED ITS 28-DAY CONCRETE STRENGTH.
- CONCRETE STREMS IT.

 REMOVE (E) 1966 RETROFIT CONCRETE ABOVE FINISHED GRADE ELEVATION. DO NOT DAMAGE (E) PERIPHERAL CONCRETE CURB OR CONCRETE STEM WALL OF (E) 1966 RETROFIT BELOW FINISHED GRADE.
- AT EACH ANCHOR LOCATION INDICATED ON PLAN. CORE DRILL 14-INCH DIAMETER VERTICAL HOLE THROUGH (E) PERIPHERAL CONCRETE CURB AND REMAINING CONCRETE STEM WALL OF (E) 1966 RETROFIT BELOW FINISHED GRADE. DO NOT OTHERWISE DAMAGE (E) PERIPHERAL CONCRETE CURB OR STEM WALL OF (E) 1966
- RETROFIT.
 INSTALL HELICAL SCREW ANCHORS.
- INSTALL ANCHOR CHAIRS.
- INSTALL ANUHOR CHAIRS.
 GROUT AROUND HELICAL SCREW ANCHOR SHAFT UP TO 3" BELOW FINISHED GRADE.
 REMOVE TOP 4" OF CONCRETE FROM (E) 1966 RETROFIT CONCRETE AND (E)
 PERIPHERAL CONCRETE CURB AND REPLACE BACK WITH NON-SHRINK GROUT.
- PROVIDE 1% SLOPE AWAY FROM TANK
- PAINT ALL EXPOSED METAL IN CONFORMANCE WITH PROJECT SPECIFICATION.













original size sheet

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ient HUMBOLDT BAY MUNICIPAL WATER DISTRICT * SAMOA RESERVOIR SEISMIC RETROFIT

2024-05-01

AS SHOWN

TYPICAL DETAILS - 2

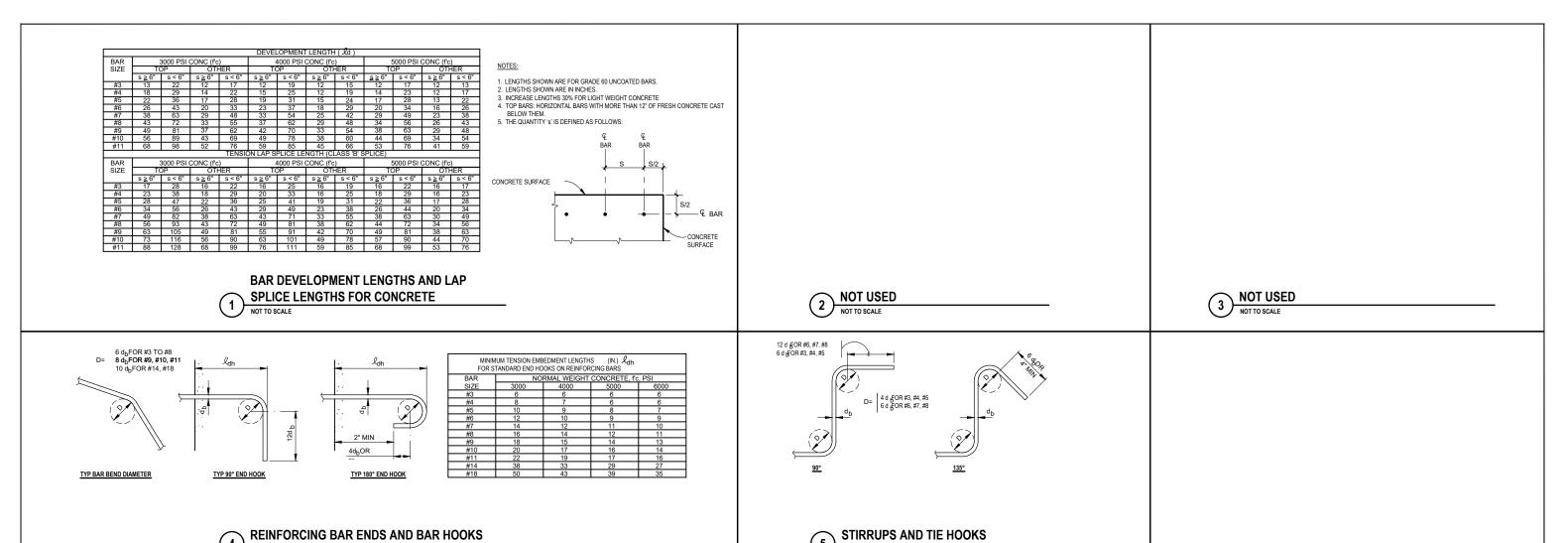
1'-33/8"

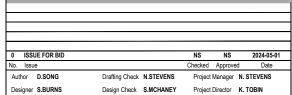
1'-33/8"

RADIAL VIEW

€ HELICAL ANCHOR

CONSTRUCTION SEQUENCING







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Client	HUMBOLDT BAY MUNICIPAL WATER
	DISTRICT
Project	SAMOA RESERVOIR SEISMIC
	RETROFIT

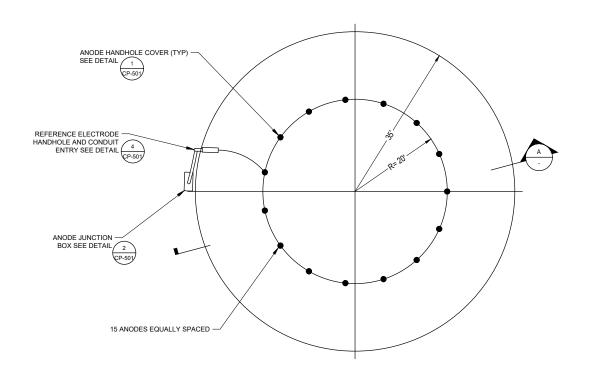
Title TYPICAL DETAILS - 3

Size ANSI D

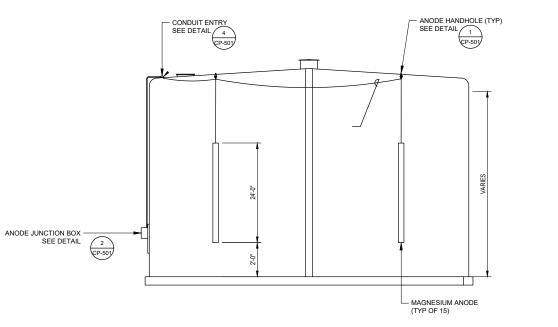
Project No.

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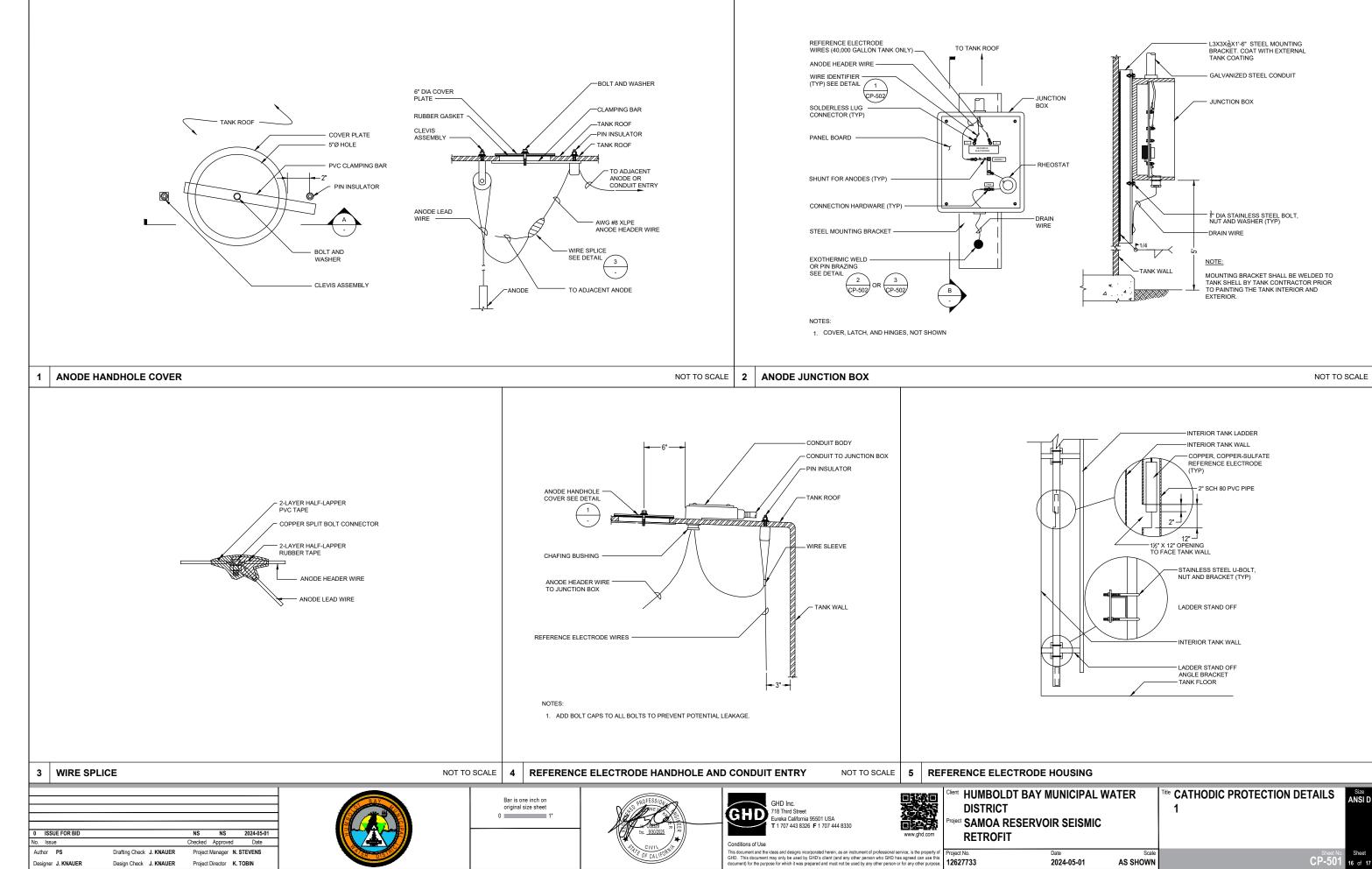


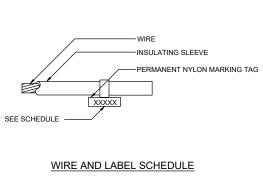
NEW CATHODIC PROTECTION - 1MG TANK PLAN SCALE: NTS



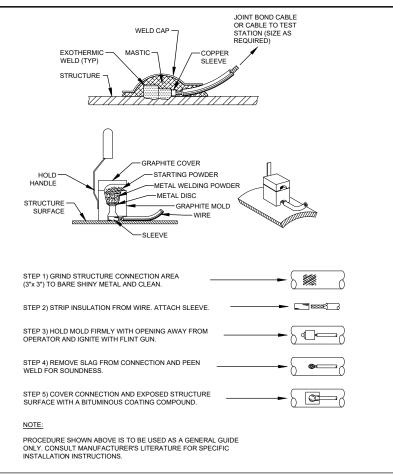
NEW CATHODIC PROTECTION - 1MG TANK SECTION SCALE: NTS

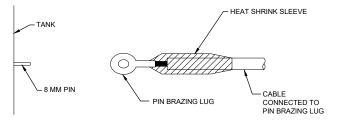






STRUCTURE	WIRE TYPE	LABEL	
ANODE HEADER WIRE	#8 XLPE	ANODE-1	
REFERENCE ELECTRODE #1	#10 RHH-RHW	REF-1	
REFERENCE ELECTRODE #2	#10 RHH-RHW	REF-2	
DRAIN WIRE	#8 HMWPE	TANK	





NOTES:

- FOR DIRECT TYPE CONNECTION, USE A 8MM DIRECT BRAZING PIN AND A PIN BRAZING CABLE LUG OR USE A 8MM THREADED PIN.
- 2. CLEAN THE METAL TO A BRIGHT FINISH.
- 3. LOAD THE BRAZING GUN WITH THE PIN AND FERRULE.
- 4. ADJUST AS NECESSARY AND BRAZE.
- 5. TEST THE CONNECTION.

1 WIRE IDENTIFIER NOT TO SCALE 2 EXOTHERMIC WELD (TYP) NOT TO SCALE 3 PIN BRAZING (TYP) NOT TO SCALE

0 ISSUE FOR BID NS NS 2024-05-01 No. Issue Checked Approved Date Author PS Drafting Check J. KNAUER Project Manager N. STEVENS Designer J. KNAUER Design Check J. KNAUER Project Director K. TOBIN



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Client HUMBOLDT BAY MUNICIPAL WATER DISTRICT * SAMOA RESERVOIR SEISMIC

2024-05-01

Title CATHODIC PROTECTION DETAILS ANSI D 2

AS SHOWN